

海岸河口水域波浪传播数值模拟^{*}

洪广文 冯卫兵 张洪生

(河海大学港口航道及海岸工程学院 南京 210098)

摘 要 提出了水深与流场缓变水域波浪传播数学模型——水流中依赖时间变量并考虑能耗的波浪“缓坡方程”及其等价的控制方程组,分析比较了无水流情况此理论模型与其相应的两种抛物型近似的差别,提供了长江口波浪变形数值模拟计算工程实例.实例表明,该模型能适应河口三角洲大范围水域波浪传播数值计算.

关键词 波浪绕射-折射;摩阻耗散;缓坡方程;抛物型近似

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海岸河口水域多变的水底地形与水流流场引起波浪传播现象复杂多变.为此,本文提出一个考虑绕射、折射、能耗和流场等多种因素影响,适用于大范围海域定常和非定常波传播的数学模型;着重分析比较无水流情况理论模式与其相应的两种抛物型近似的差别,论证后二者存在的缺陷;作为工程应用,提供河口三角洲大范围水域波浪变形数值模拟实例.

1 水流中波浪传播数学模型

关于水流中波浪传播问题,目前文献[1~3]已提出3个略有差别的线性理论模式.笔者分别采用分离变量法和变分法推导出另一个考虑能耗影响的缓变水深与流场中非定常波传播理论模型^[4,5],其表达式可概括如下:

- a. 非定常波浪“缓坡方程”
$$\frac{\text{D}}{\text{D}t}(\frac{\text{D}}{\text{D}t} + W^*)\Phi + (\nabla \cdot \boldsymbol{U})\frac{\text{D}}{\text{D}t}\Phi + W^*\Phi - \nabla \cdot (\tilde{C}\tilde{C}_g \nabla \Phi) + (\tilde{\sigma}^2 - k^2\tilde{C}\tilde{C}_g)\Phi = 0 \tag{1}$$
- b. 波数守恒方程
$$\frac{\partial \boldsymbol{K}}{\partial t} + \nabla \omega = 0 \tag{2}$$
- c. 波数矢无旋性方程
$$\nabla \times \boldsymbol{K} = 0 \tag{3}$$
- d. 绝对频率表达式
$$\omega = -\frac{\partial \Psi}{\partial t} = \sigma + \boldsymbol{U} \cdot \boldsymbol{K} \tag{4}$$

式中:

波动速度势

$$\varphi(x,y,z,t) = \frac{\text{ch}k(h+z)}{\text{ch}kh}\Phi(x,y,t) \tag{5}$$

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第一作者简介:洪广文,男,教授,海岸工程专业.

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$$\Phi(x, y, t) = -igR(x, y, t)e^{i\Psi(x, y, t)} \quad (6)$$

$$\eta(x, y, t) = ae^{i\Psi} \quad (7)$$

$$\sigma = -\frac{D}{Dt}\Psi$$

$$\sigma^2 = \tilde{\sigma}^2 - \frac{1}{4}W^{*2} \quad (8)$$

$$\tilde{\sigma}^2 = gkh, \tilde{C} = \tilde{\sigma}/k, \tilde{C}_g = \frac{\tilde{C}}{2}\left(1 + \frac{2kh}{sh^2kh}\right) \quad (9)$$

$$\mathbf{K} = \nabla \Psi$$

$$\frac{D}{Dt} = \frac{\partial}{\partial t} + \mathbf{U} \cdot \nabla$$

式中: $a = \sigma R \left[1 + \left(\frac{1}{\sigma R} \frac{DR}{Dt} + \frac{W^*}{\sigma} \right)^2 \right]^{1/2} \approx \sigma R$ ——波面振幅; W^* ——能耗系数; $\mathbf{U}(x, y, t)$ ——给定水流水平速度矢; $h(x, y)$ ——水域水深; g ——重力加速度。

1.1 水流中“缓坡方程”的等价方程组

将式(6)代入式(1)并分离实部与虚部可得式(1)的等价方程组,即波作用守恒方程

$$\frac{\partial A}{\partial t} + \nabla \cdot \left(\mathbf{U} + \frac{\tilde{C}\tilde{C}_g}{\sigma} \mathbf{K} \right) A = W^* A \quad A = \sigma R^2 \quad (10)$$

和光程函数方程

$$K^2 = k^2 + \frac{1}{\tilde{C}\tilde{C}_g R} \left\{ \nabla \cdot (\tilde{C}\tilde{C}_g R) - \left[\frac{D}{Dt} \left(\frac{D}{Dt} + W^* \right) R + \left(\nabla \cdot \mathbf{U} \right) \left(\frac{D}{Dt} + W^* \right) R + \frac{R}{4} W^{*2} \right] \right\} \quad (11)$$

此方程组式(10)及(11)可用于替代不便于工程应用计算的椭圆型方程式(1)。

1.2 水流中定常波传播理论模型

对于工程应用中常见的定常情况,

$$\Phi(x, y, t) = \Phi(x, y)e^{-i\omega t} \quad \mathbf{U}(x, y, t) = \mathbf{U}(x, y)$$

上述式(1)及(10)及(11)可化简为

$$\nabla \cdot [\tilde{C}\tilde{C}_g \nabla \Phi - (\mathbf{U} \cdot \nabla \Phi + W^* \Phi) \mathbf{U}] + 2i\omega \mathbf{U} \cdot \nabla \Phi + [k^2 \tilde{C}\tilde{C}_g + \omega^2 - \tilde{\sigma}^2 + i\omega(\nabla \cdot \mathbf{U} + W^*)] \Phi = 0 \quad (12)$$

$$\nabla \cdot A \left[\mathbf{U} + \frac{\tilde{C}\tilde{C}_g}{\sigma} \mathbf{K} \right] = -W^* A \quad (13)$$

$$K^2 = k^2 + \frac{1}{\tilde{C}\tilde{C}_g R} \left\{ \nabla \cdot [\tilde{C}\tilde{C}_g \nabla R - (\mathbf{U} \cdot \nabla R + W^* R) \mathbf{U}] - \frac{R}{4} W^{*2} \right\} \quad (14)$$

$$\omega = \sigma + \mathbf{K} \cdot \mathbf{U} = \text{const} \quad (15)$$

1.3 无流水域非定常波传播数学模型

$$\frac{\partial}{\partial t} \left(\frac{\partial}{\partial t} + W^* \right) \Phi - \nabla \cdot (\tilde{C}\tilde{C}_g \nabla \Phi) + (\tilde{\sigma}^2 - k^2 \tilde{C}\tilde{C}_g) \Phi = 0 \quad (16)$$

$$\frac{\partial}{\partial t} (H^2) + \nabla \cdot (H^2 \tilde{C}_g \frac{\mathbf{K}}{k}) = -W^* H^2 \quad (17)$$

和

$$K^2 = k^2 + \frac{1}{\tilde{C}\tilde{C}_g H} \left\{ \nabla \cdot (\tilde{C}\tilde{C}_g H) - \left[\frac{\partial^2 H}{\partial t^2} + W^* \frac{\partial H}{\partial t} + \frac{H}{4} W^{*2} \right] \right\} \quad (18)$$

$$\begin{aligned}\nabla \times \mathbf{K} &= 0 \\ \frac{\partial \mathbf{K}}{\partial t} + \nabla \omega &= 0 \\ \omega^2 &= \bar{\sigma}^2 - \frac{1}{4} W^{*2} \text{ 或 } \bar{\sigma}^2 = \omega^2 \left(1 + \frac{W^{*2}}{4\omega^2} \right) \approx \omega^2 \\ W^* &= \frac{4f_w H}{3\pi g} \left(\frac{K\omega}{k \sinh kh} \right)^3\end{aligned}\quad (19)$$

式中 f_w 为底摩阻系数.

当 $W^* = 0$ 时, 式(16)即化为文献[6]的理论模式.

1.4 无水流水域定常波传播数学模型

$$\nabla (\tilde{C}\tilde{C}_g \nabla \Phi) + (k^2 \tilde{C}\tilde{C}_g - \frac{1}{4} W^{*2} + i\omega W^*)\Phi = 0 \quad (20)$$

$$\nabla (H^2 \tilde{C}_g \frac{\mathbf{K}}{k}) = -W^* H^2 \quad (21)$$

和

$$K^2 = k^2 + \frac{1}{\tilde{C}\tilde{C}_g R} \left\{ \nabla (\tilde{C}\tilde{C}_g \nabla H) - \frac{H}{4} W^{*2} \right\} \quad (22)$$

$$\omega = \text{const} \quad (2')$$

$$\nabla \times \mathbf{K} = 0 \quad (3')$$

当

$$\frac{\nabla H}{kH} \ll 1 \text{ 与 } \left(\frac{W^*}{\omega} \right) \ll 1$$

则

$$K = k$$

$$\nabla \times \mathbf{k} = 0$$

$$\nabla (H^2 \tilde{C}_g \frac{\mathbf{k}}{k}) = -W^* H^2 \quad (23)$$

此即纯折射控制方程.

当

$$\frac{\nabla \tilde{C}\tilde{C}_g}{k\tilde{C}\tilde{C}_g} \ll 1 \text{ 与 } \left(\frac{W^*}{\omega} \right) \ll 1$$

则

$$K^2 = k^2 + \frac{1}{H} \nabla \cdot \nabla H \quad (24)$$

$$\nabla \times \mathbf{K} = 0$$

$$\nabla (H^2 \tilde{C}_g \frac{\mathbf{K}}{k}) = -W^* H^2$$

此即纯绕射控制方程.

当 $W^{*2} \ll 1$ 时, 式(20)化为文献[7]的理论模式; 当 $W^* = 0$ 时, 式(20)即为著名的缓坡方程式^[8].

1.5 定常波传播抛物型近似 I

类似文献[7,9], 令 $\Phi = \Phi_i + \Phi_r$, 并假定入射波复振幅 Φ_i 与反射波复振幅 Φ_r 按以下方式辟分(取 x 轴为主波向):

$$\Phi_i = \frac{1}{2} \Phi - \frac{i}{2k_x} \Phi_x \quad \Phi_r = \frac{1}{2} \Phi + \frac{i}{2k_x} \Phi_x \quad (25)$$

略去反射波, 由式(20)可得抛物型近似:

$$\Phi_x = \left\{ \frac{i}{2} \left[k_x + \frac{k^2}{k_x} \left(1 + \frac{W^{*2}}{4k^2 \tilde{C}\tilde{C}_g} \right) \right] - \frac{\omega W^*}{\tilde{C}\tilde{C}_g k_x} - \frac{(k_x \tilde{C}\tilde{C}_g)_x}{2k_x \tilde{C}\tilde{C}_g} \right\} \Phi + \frac{i}{2k_x \tilde{C}\tilde{C}_g} (\tilde{C}\tilde{C}_g \Phi_y)_y \quad (26)$$

将式(6)代入上式可得控制方程组

$$(H^2 \bar{C}_g \frac{k_x}{k})_x + (H^2 \bar{C}_g \frac{K_y}{k})_y = -W^* H^2 \quad (27)$$

和

$$\frac{1}{\bar{C}\bar{C}_g H} (\bar{C}\bar{C}_g H_y)_y + \left[k^2 - \frac{W^{*2}}{4\bar{C}\bar{C}_g} - K_y^2 - k_x(2K_x - k_x) \right] = 0 \quad (28)$$

如令

$$\Phi = (-ig) \bar{R}(x, y) e^{ik_0 x} \quad \bar{R} = \bar{a}/\omega \quad (29)$$

则式(26)可化为另一种抛物型近似:

$$2ik_x \bar{C}\bar{C}_g \bar{R}_x + 2k\bar{C}\bar{C}_g \left\{ k \left[\frac{1}{2} \left(1 + \frac{W^{*2}}{4k^2 \bar{C}\bar{C}_g} + \frac{i\omega W^*}{k^2 \bar{C}\bar{C}_g} \right) + \frac{1}{k^2} k_x \left(\frac{1}{2} k_x - k_{0x} \right) \right] + \right. \\ \left. (k_x \bar{C}\bar{C}_g)_x \right\} \bar{R} + (\bar{C}\bar{C}_g \bar{R}_y)_y = 0 \quad (30)$$

式中: $\bar{R}$ ——复函数; k_0 ——入射波波数.

1.6 定常波传播抛物型近似 II

当取^[10]

$$\Phi_i = \frac{1}{2} \Phi - \frac{i}{2k} \Phi_x \quad \Phi_r = \frac{1}{2} \Phi + \frac{1}{2k} \Phi_x$$

则式(20)可化为另一种抛物型近似:

$$\Phi_x = \left\{ ik \left[1 + \frac{W^{*2}}{8k^2 \bar{C}\bar{C}_g} + i \frac{\omega W^*}{2k^2 \bar{C}\bar{C}_g} \right] - \frac{1}{2k\bar{C}\bar{C}_g} (k\bar{C}\bar{C}_g)_x \right\} \Phi + \frac{i}{2k\bar{C}\bar{C}_g} (\bar{C}\bar{C}_g \Phi_y)_y \quad (31)$$

将式(6)代入式(31), 则式(31)可化为控制方程组

$$(H^2 \bar{C}_g)_x + (H^2 \bar{C}_g \frac{K_y}{k})_y = -W^* H^2 \quad (32)$$

和

$$\frac{1}{\bar{C}\bar{C}_g H} (\bar{C}\bar{C}_g H_y)_y + \left\{ k^2 - \frac{W^{*2}}{4\bar{C}\bar{C}_g} - K_y^2 - k(2K_x - k) \right\} = 0 \quad (33)$$

如将式(29)代入式(31), 则有另一形式的抛物型近似:

$$2ik\bar{C}\bar{C}_g \bar{R}_x + 2k\bar{C}\bar{C}_g \left[k \left(1 + \frac{W^{*2}}{8k^2 \bar{C}\bar{C}_g} + i \frac{\omega W^*}{2k^2 \bar{C}\bar{C}_g} \right) - k_0 \right] \bar{R} + (k\bar{C}\bar{C}_g)_x \bar{R} + (\bar{C}\bar{C}_g \bar{R}_y)_y = 0 \quad (34)$$

1.7 波浪联合绕射、折射解析-数值解

设 S 为联合绕射、折射波向线, l 为其正交线(波峰线), α 为波向线的切向与 x 轴的夹角, 利用坐标变换

$$\frac{\partial}{\partial S} = \cos \alpha \frac{\partial}{\partial x} + \sin \alpha \frac{\partial}{\partial y} \quad \frac{\partial}{\partial l} = -\sin \alpha \frac{\partial}{\partial x} + \cos \alpha \frac{\partial}{\partial y}$$

则联合绕射、折射波能守恒方程(21)可改写为

$$(H^2 C_g b \frac{K}{k})^{3/2} \frac{d}{dS} (H^2 C_g b \frac{K}{k}) = -\beta \quad (35)$$

$$\text{式中: } \beta = \frac{4f_w}{3\pi g} \left(\frac{Kb}{k} \right)^{1/2} \left(\frac{\omega}{\sqrt{C_g \sinh kh}} \right)^3 \left(\frac{b}{b_*} \right)^{1/2} = \exp \left\{ -\frac{1}{2} \int_{S_*}^S \frac{\partial \alpha}{\partial l} dS \right\}$$

式(35)的解析解为

$$H = K_s K_{rd} K_j H_* \quad (36)$$

$$K_s = (C_g^*/C_g)^{1/2} \quad (37)$$

$$K_{rd} = \left(\frac{K}{k} b \right)^{1/2} / \left(\frac{K}{k} b \right)^{1/2} \quad (38)$$

$$K_f = \left\{ 1 + \frac{2H_* \omega^3}{3\pi g (KC_g/k)_*} \int_{S_*}^S f_w K_{rd} \left(\frac{K_s}{shkh} \right)^3 dS \right\}^{-1} \quad (39)$$

$$\frac{\partial \alpha}{\partial S} = \frac{1}{K} \frac{\partial K}{\partial l} \quad (40)$$

对于平行于 y 轴的直线型等深线特例,

$$K \sin \alpha = (K \sin \alpha)_* \quad (41)$$

$$\left(\frac{b_*}{b} \right)^{1/2} = \left(\frac{\cos \alpha_*}{\cos \alpha} \right)^{1/2} \quad (42)$$

$$K_f = \left\{ 1 + \frac{2H_* \omega^3}{3\pi g (KC_g \cos \alpha / k)_*} \int_{x_*}^x f_w \left(\frac{K_s K_{rd}}{shkh} \right)^3 dx \right\}^{-1} \quad (43)$$

未知波数矢 K (或 K 与 α) 则须由波数矢守恒方程 (式 (40) 或式 (3)) 和光程函数方程式 (22) 及 (36) 联合迭代数值计算来确定。

1.8 理论模式及其近似式的比较分析

以上分别给出了波浪联合绕射、折射理论模式原型及其两种基本抛物型近似和单纯折射近似。为了分析这些近似模式存在的缺陷, 现将各模式控制方程归纳如下:

能量守恒方程

$$\text{原型} \quad \left[\left(H^2 C_g \frac{K_x}{k} \right)_x \right] + \left(H^2 C_g \frac{K_y}{k} \right)_y = - W^* H^2 \quad (21')$$

$$\text{抛物型 I} \quad \left[\left(H^2 C_g \frac{k_x}{k} \right)_x \right] + \left(H^2 C_g \frac{K_y}{k} \right)_y = - W^* H^2 \quad (27')$$

$$\text{抛物型 II} \quad [(H^2 C_g)_x] + \left(H^2 C_g \frac{K_y}{k} \right)_y = - W^* H^2 \quad (32')$$

$$\text{折射模型} \quad \left[\left(H^2 C_g \frac{k_x}{k} \right)_x \right] + \left[\left(H^2 C_g \frac{k_y}{k} \right)_y \right] = - W^* H^2 \quad (44)$$

光程函数方程

$$\text{原型} \quad \frac{1}{CC_g H} \{ (CC_g H_y)_y + [(CC_g H_x)_x] \} + \left\{ k^2 - \frac{W^{*2}}{4CC_g} - K_y^2 - [K_x^2] \right\} = 0 \quad (22')$$

$$\text{抛物型 I} \quad \frac{1}{CC_g H} \{ (CC_g H_y)_y \} + \left\{ k^2 - \frac{W^{*2}}{4CC_g} - K_y^2 - [k_x (2K_x - k_x)] \right\} = 0 \quad (28')$$

$$\text{抛物型 II} \quad \frac{1}{CC_g H} \{ (CC_g H_y)_y \} + \left\{ k^2 - \frac{W^{*2}}{4CC_g} - K_y^2 - [k (2K_x - k)] \right\} = 0 \quad (33')$$

$$\text{折射模型} \quad [k^2 = K^2] \quad (45)$$

由上述各模式对比分析可知, 带方括号的项系各近似式与原型准确式差别所在。也就是说, 抛物型近似 I 假定 $K_x = k_x$, 抛物型 II 假定 $K_x = k$, 而折射模型则取 $K_x = k_x$, $K_y = k_y$ (即 $K = k$)。因此, 抛物型 I 比抛物型 II 及折射模型具有较好的近似性, 而抛物型 II 则应限于入射波小角度入射和波浪传播时波向变化小的计算情况 (x 轴应指向主波向)。例如, 平行直线型等深线情况, 不考虑能耗的波高变化如下:

$$\text{原型} \quad H^2 C_g K_x / k = \text{const}$$

$$\text{I 型及折射} \quad H^2 C_g k_x / k = \text{const}$$

II 型

$$H^2 C_g = \text{const}$$

可见,此种特殊情况抛物型近似 II 只计及波浪因水深变化所产生的变形,并没有反映绕射、折射的影响,而抛物型 I 则考虑了变形与折射的影响.

2 边界条件

2.1 起始边界条件

对于控制方程组式(21)(22)(2') (3),起始边界条件可按外海有效波高 H_{s*} 、周期 T_* 、波向 α_* 及水深 h_* 来给定.

2.2 固体界面边界条件

考虑固体界面具有不完全反射特性(反射系数 K_r 及相位差 ϵ_r)其边界条件为第三类(Robin)边界条件^[7,11]

$$\frac{\partial \varphi}{\partial n} + B\varphi = 0 \quad (46)$$

式中:

$$B = B_R + iB_I = -\sin\alpha^* \frac{1-R}{1+R} \quad (47)$$

$$R = K_r e^{i\epsilon_r}$$

$$B_R = K \sin\alpha^{**} \frac{2K_r \sin\epsilon_r}{1 + 2K_r \cos\epsilon_r + K_r^2}$$

$$B_I = K \sin\alpha^{**} \frac{K_r^2 - 1}{1 + 2K_r \cos\epsilon_r + K_r^2}$$

将式(5)(6)代入式(46),即得确定固体界面上波高和波向的边界条件

$$\frac{\partial H}{\partial n} = -B_R H \quad (48)$$

和

$$\frac{\partial \Psi}{\partial n} = K_n \text{ 或 } \alpha^* = \theta^* + \arcsin(-B_I/K) \quad (49)$$

式中: n ——界面外法线; θ^* ——界面切线方向角; α^* ——界面波向; α^{**} ——入射波波向与界面切向的夹角.

2.3 开边界条件

当计算大范围水域波浪传播变形时,为了简化计算,往往必须人为给定侧边界或上游边界.对此,人为虚拟边界要求波浪能自由通过而不致产生反射波干涉计算域内波浪传播,因此可按式(46)或式(46)(47)取 $K_r = 0$ 或 $K_r \ll 1$ ($\epsilon_r = \pi$) 来确定开边界条件,即

$$\frac{\partial H}{\partial n} = 0 \quad (48')$$

和

$$\alpha^* = \theta^* + \alpha^{**} \quad (49')$$

3 控制方程非正交坐标变换

3.1 非正交直线坐标

对于工程中常见的折线型或可简化为多段折线型水域、建筑物边界,可采用下述非正交直线坐标进行坐标变换,以便准确满足边界条件,改善数值模拟精度.为此,采用一非直角坐标系 u, v (图1)将 x, y 直角坐标系中的两折线内水域(物理域)变换为 u, v 坐标系中的矩形域(数学域).因此, $v=0$ 和 $v=y_0$ 分别为原物理域右、左边界的映射.由于任一直线 $v=v_0$ 上任一点 $u=u_0$ 和 $u=0$ (口门处)与 u 轴距离对该点处左、右边界

距离之比相等,即

$$\frac{y-y_0}{y_b}=\frac{y-x\tan\theta_2-y_0}{y_b+x(\tan\theta_2-\tan\theta_1)}$$

因此所需的非直角坐标系

$$\begin{cases} u=x \\ v=\frac{y-A-Bx}{1+Dx} \end{cases} \text{ 或 } \begin{cases} x=u \\ y=A+v+(B+D)u \end{cases}$$

(50)

式中: $A=y_0$; $B=\tan\theta_1$; $D=(\tan\theta_2-\tan\theta_1)/y_b$;
 y_b ——口门宽度; θ_1,θ_2 ——右、左边界的夹角;
 $|\theta_1|,|\theta_2|<90^\circ$.

3.2 控制方程与边界条件的变换

利用上述非直角坐标系式(50)可将前述控制方程组式(3)(21)(22)与边界条件式(48)(49)变换为如下表达式:

波数矢无旋性方程

$$\frac{\partial K\sin\alpha}{\partial u}-\frac{B+Dv}{1+Du}\frac{\partial K\sin\alpha}{\partial v}-\frac{1}{1+Du}\frac{\partial K\cos\alpha}{\partial v}=0$$

(51)

波能守恒方程

$$\frac{\partial}{\partial u}\left(H^2C_g\cos\alpha\frac{K}{k}\right)-\frac{B+Dv}{1+Du}\frac{\partial}{\partial v}\left(H^2C_g\cos\alpha\frac{K}{k}\right)+\frac{1}{1+Du}\frac{\partial}{\partial v}\left(H^2C_g\sin\alpha\frac{K}{k}\right)=-W^*H^2$$

(52)

光程函数方程

$$\begin{aligned} K^2=k^2+\frac{1}{H}\Big\{\frac{\partial^2H}{\partial u^2}+\frac{1+(B+Dv)^2}{1+Du}\frac{\partial^2H}{\partial v^2}+\frac{2D(B+Dv)}{(1+Du)^2}\frac{\partial H}{\partial v}- \\ \frac{2(B+Dv)^2}{1+Du}\frac{\partial^2H}{\partial u\partial v}\Big\}+\frac{1}{HCC_g}\Big\{\Big[\frac{\partial CC_g}{\partial u}-\frac{B+Dv}{1+Du}\frac{\partial CC_g}{\partial v}\Big]\frac{\partial H}{\partial u}+ \\ \Big[-\frac{B+Dv}{1+Du}\frac{\partial CC_g}{\partial u}+\frac{1+(B+Dv)^2}{(1+Du)^2}\frac{\partial CC_g}{\partial v}\Big]\frac{\partial H}{\partial v}\Big\}-\frac{W^{*2}}{4CC_g} \end{aligned}$$

(53)

固体界面边界条件($v=0,\theta=\theta_1;v=y_b,\theta=\theta_2$)为

$$\left\{\sin\theta_i\left[\frac{\partial H}{\partial u}-\frac{B+Dv}{1+Du}\frac{\partial H}{\partial v}\right]-\frac{\cos\theta_i}{1+Du}\frac{\partial H}{\partial v}\right\}+B_RH=0\quad\theta_i=\theta_1,\theta_2$$

(54)

和

$$\alpha^{**}=\theta_i-\arcsin(B_l/K)$$

4 工程应用

上述控制方程的差分格式、迭代计算模式和数值模拟结果与解析解及室内物模试验验证结果见文献[12].本文提供了应用此理论模式及计算方法计算的天然情况下长江口北槽附近水域和深水航道一期导堤工程水域内(利用非正交坐标变换)波浪传播变形成果.其计算范围约60 km×60 km和20 km×20 km,部分计算结果见图2~4.图2、3分别为长江口北槽中附近大范围水域波高、波向矢量变化(矢量长度表示波高大小,矢量方向表示波向)和相应的等波高线分布.由图可见,水底地形的复杂变化会引起波高、波向的沿程变化.图4为反射系数 K_r 按导堤不同结构参照专门室内试验结果(南堤分段取0.55,0.45,北堤分段取0.45,

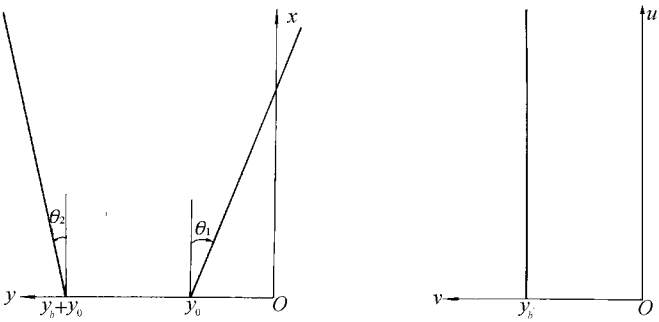


图 1 非正交直线坐标变换示意图
Fig.1 Non-orthogonal coordinate conversion

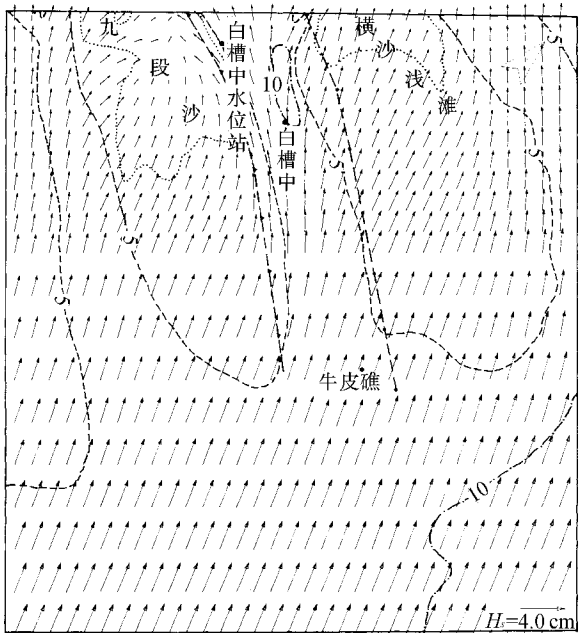


图2 波高、波向矢量图(波浪 $H_s^* = 3.31\text{ m}$,
 $T = 6.3\text{ s}$ 波向 SSE 潮位 4.44 m)
Fig.2 Wave height and direction arrows in large estuarine
water area($H_s^* = 3.31\text{ m}$, $T = 6.3\text{ s}$, $\Delta = 4.44\text{ m}$)

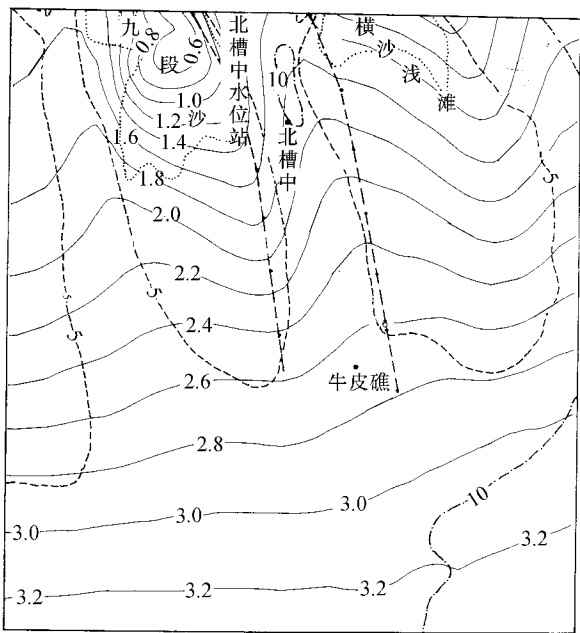


图3 波高等值线(波浪 $H_s^* = 3.31\text{ m}$,
 $T = 6.3\text{ s}$ 波向 SSE 潮位 4.44 m)
Fig.3 Wave height contours for numerical simulation
in Fig.2 $H_s^* = 3.31\text{ m}$, $T = 6.3\text{ s}$, $\Delta = 4.44\text{ m}$)

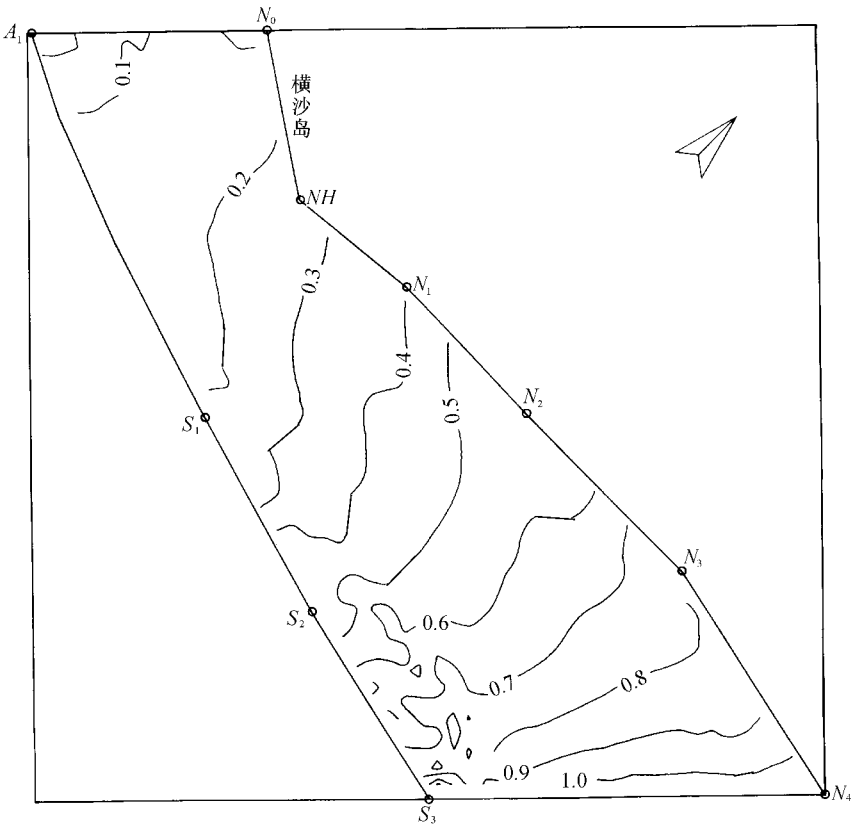


图4 导堤内侧波高等值线($H_s^* = 3.80\text{ m}$, $T = 6.2\text{ s}$ 潮位 2.00 m)
Fig.4 Wave height contours for numerical simulation inside submerged jetties($H_s^* = 3.80\text{ m}$, $T = 6.2\text{ s}$, $\Delta = 2.00\text{ m}$)

0.53 0.50)及取反射相位差 $\epsilon_r = 0$ 等条件组合所计算的导堤内水域波高变化的等波高分布.由图可见,外海波高经长约 60 km 传播至深水航道内水域波高明显衰减.从多达 20 种各种波要素与水位组合条件下计算结果可知,本文所提出的理论模式综合考虑了绕射、折射与摩阻耗散等因素的影响,能够对诸如长江口水域复杂地形及建筑物大范围波浪传播变形计算提供合理可靠的结果.

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Numerical Modelling of Wave Propagation in Water of
Varying Topography and Current

Hong Guangwen Feng Weibing Zhang Hongsheng

(College of Harbor ,Waterway and Coastal Engineering ,Hohai Univ. ,Nanjing 210098)

Abstract A numerical model for wave propagation in water of varying topography and current is proposed ,and time-dependent wave mild-slope equation with a dissipation term and corresponding equivalent governing equations are presented.Two different expressions of parabolic approximations for the case of the absence of current are also given and analyzed.Examples of numerical simulation for wave transformation in large estuarine water areas are provided.

Key words diffraction-refraction ;dissipation ;time-dependent mild slope equation ;parabolic approximation